

# CHAPTER 3

## VIBRATION THEORY

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## SINGLE DEGREE OF FREEDOM SYSTEMS (SDOF)

Many simple structures can be idealized as a concentrated or lumped mass,  $m$ , supported by a massless structure with stiffness,  $k$ , in the lateral direction. A one story building or structural frame as shown in Fig 3.1 has a heavy and stiff/rigid roof. When this building is subjected to lateral load,  $F$ , it has only one degree of dynamic freedom, the lateral sway or displacement,  $\Delta$ , as indicated on the figure. An equivalent dynamic model of this building consists of a single column with equivalent stiffness,  $k$ , supporting a lumped mass of magnitude,  $m$ .

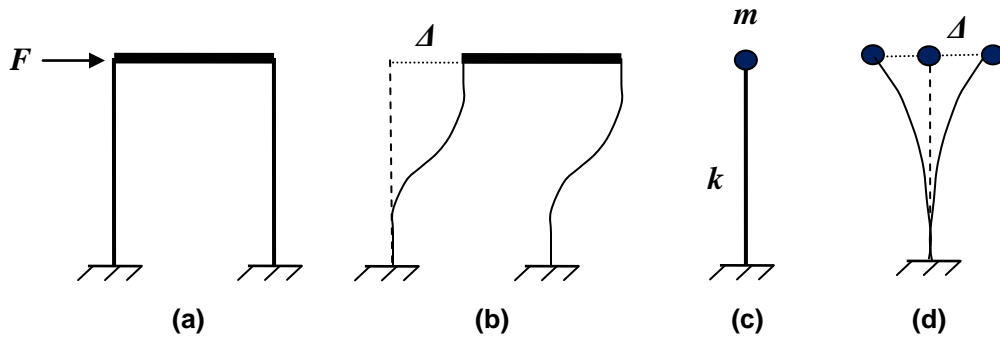


Fig. 3.1 Single Degree of Freedom Systems

## STIFFNESS, $K$

Stiffness is the force required to produce a unit displacement in the direction of the force (kips/in, lbs/in).

$$K = F/\Delta$$

When there are multiple columns (members with individual stiffness), the total equivalent stiffness of the SDOF system can be summed from individual member stiffness either in parallel or in series or a combination thereof.

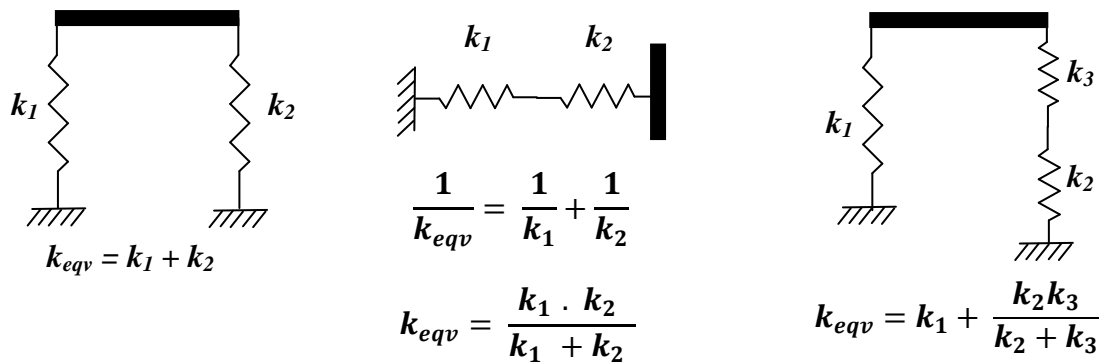


Fig. 3.2 In-parallel and In-Series Equivalent Stiffness

The inverse of stiffness is flexibility. Thus, it is the deflection produced by a unit force (in/kips, in/lbs).

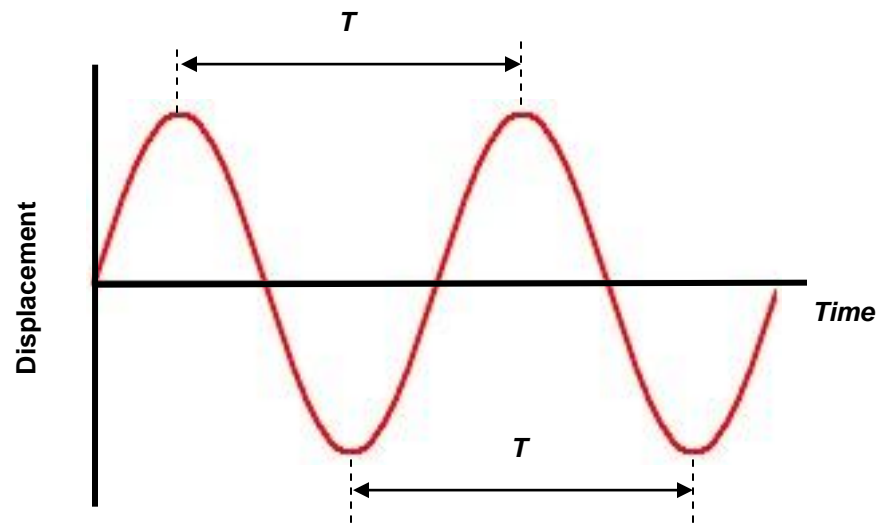
### MEMBER STIFFNESS (MEMBER RIGIDITY)

The member stiffness is a function of the length of the member,  $L$ , second moment of inertia,  $I$ , Material's Young modulus of elasticity,  $E$ , Members' cross sectional area,  $A$  (only in the case of axial stiffness), and ends condition (free, pinned, fixed). Member stiffness is also termed member rigidity. Table 3.1 summarizes stiffness expressions for different end conditions. The member's maximum deflection equations (Force/stiffness) are also shown in the Table.

$$\text{stiffness (rigidity)} = \text{force} / \text{deflection}$$

### NATURAL PERIOD

If the mass of the SDOF system shown on Fig 3.1.d is subjected to an initial displacement and then released, free vibration occurs about the static position producing a harmonic sinusoidal wave (Fig. 3.3). The time required to complete a full cycle of vibration is the natural period of vibration,  $T$ . It is the time between two successive peaks or valleys as shown on Fig. 3.3. Natural period is sometimes referred to as the *fundamental period*.



**Fig. 3.3 Natural Period of Vibration**

Natural period of vibration,  $T$ , (in sec) is a function of the mass,  $m$ , and stiffness,  $k$ .

$$T = 2\pi \sqrt{\frac{m}{k}} \quad (\text{sec})$$

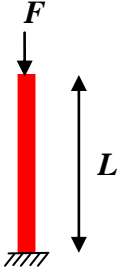
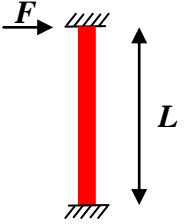
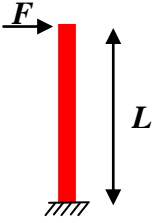
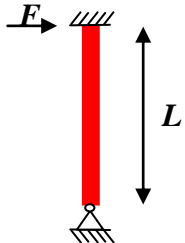
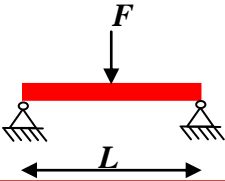
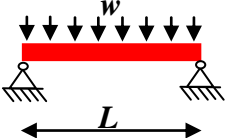
Where  $m = W/g$

$W$  = weight of the structure.

$g$  = gravitational acceleration = 32.2 ft/sec<sup>2</sup> = 386 in/sec<sup>2</sup>.

- The form of this expression indicates that the natural period increases as the mass of the system increases. The natural period also increases as the stiffness of the system decreases.

**Table 3.1 Member Stiffness for Different Boundary Conditions**

| <i>MEMBER</i>   | <i>STIFFNESS</i>     | <i>DEFLECTION</i>     |
|---|----------------------|-----------------------|
|    | $\frac{EA}{L}$       | $\frac{FL}{EA}$       |
|    | $\frac{12EI}{L^3}$   | $\frac{FL^3}{12EI}$   |
|   | $\frac{3EI}{L^3}$    | $\frac{FL^3}{3EI}$    |
|  | $\frac{3EI}{L^3}$    | $\frac{FL^3}{3EI}$    |
|  | $\frac{48EI}{L^3}$   | $\frac{FL^3}{48EI}$   |
|  | $\frac{384EI}{5L^3}$ | $\frac{5wL^4}{384EI}$ |

## NATURAL FREQUENCY

The invert of natural period,  $T$ , is the natural **linear** frequency,  $f$ , expressed in Hertz (cycle/second)

$$f = 1/T \quad (\text{Hz})$$

## ANGULAR NATURAL FREQUENCY

The angular or circular natural frequency,  $\omega$ , (also known as angular velocity) is defined as the rate of change of angular displacement (during rotation). The angular frequency is measured in radians per second and is given by:

$$\omega = 2 \pi f = 2 \pi / T = \sqrt{\frac{k}{m}} = \sqrt{\frac{k g}{W}} \quad (\text{rad/sec})$$

## STRUCTURAL DAMPING

The SDOF system shown in Fig 3.1 (d) will oscillate indefinitely, if there is nothing to dampen the harmonic motion. In practice, the internal friction of the system will resist the motion causing the vibration to die out eventually through many cycles of decaying amplitude of vibration.

The frictional resistance, or damping,  $B$ , dissipates the energy of the system, primarily through friction.

- Damping is proportional to the velocity of the vibrating system and can take the form of external or internal viscous damping, body friction damping, radiation damping, and hysteretic damping

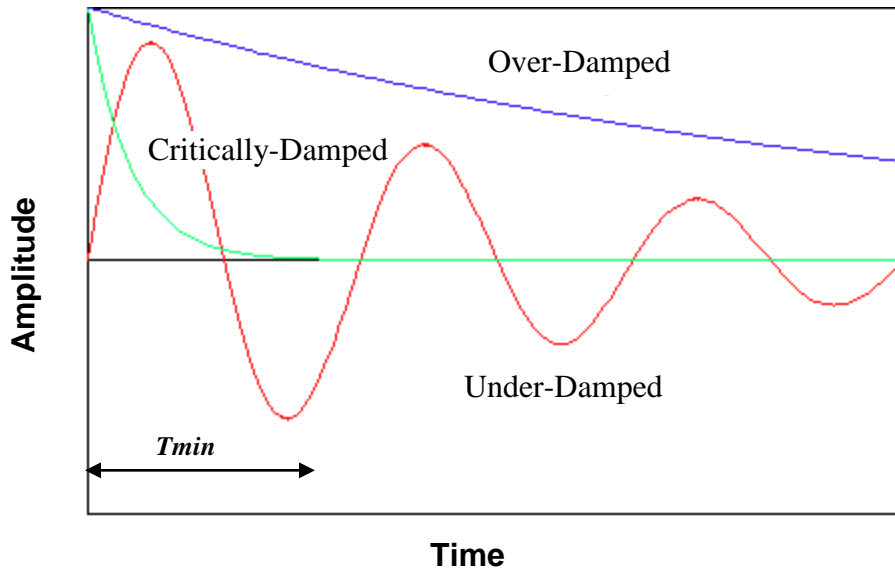
## CRITICAL DAMPING AND DAMPING RATIO

Critical damping,  $B_{critical}$ , is the amount of damping that brings the system to a static position (equilibrium) in the shortest time (see Fig. 3.4). Both *underdamped* and *overdamped* motions bring the system back to static position after a long time.

- The ratio of the actual damping of the system to the critical damping is termed damping ratio,  $\beta$ .

$$\beta = B / B_{critical}$$

Typical values of damping ratio range from 2% for welded steel structures, 5% for concrete structures, 10% for masonry shear walls, and 15% for wood structures.



**Fig. 3.4 Definition of Critical Damping**

It is to be noted that the natural period of vibration of a damped system is approximately the same as a natural period of vibration for free vibration system (undamped vibration), for the typical range of damping ratio in structural applications.

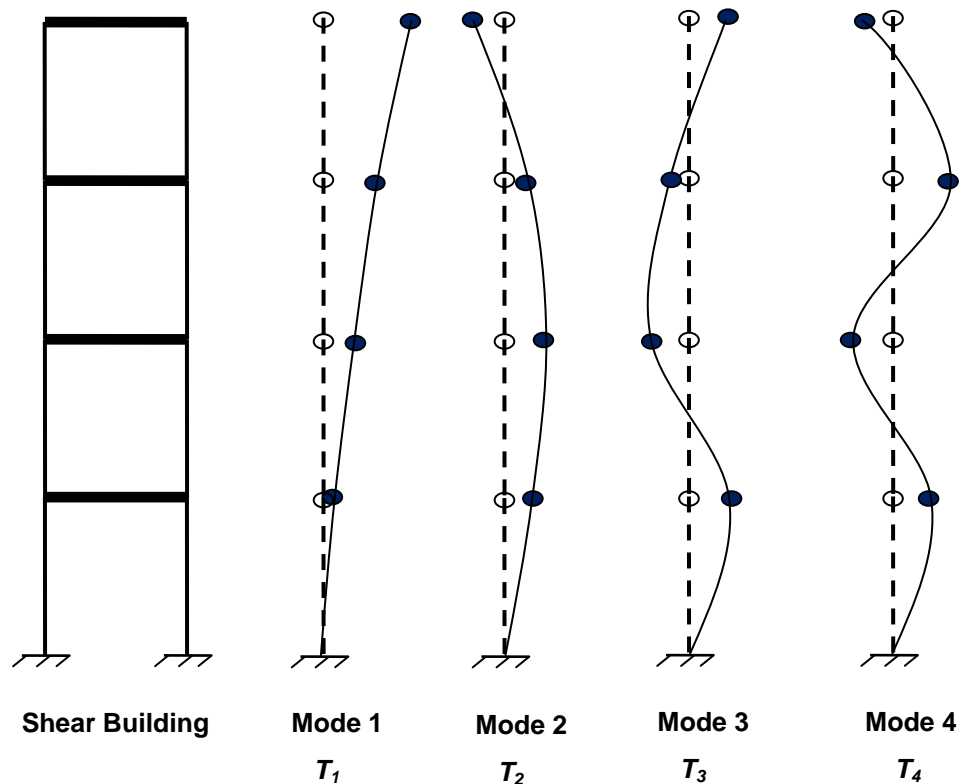
### **MULTIPLE DEGREES OF FREEDOM SYSTEMS (MDOF)**

The multi story structure shown in Fig 3.5 (a) may be idealized by assuming the mass of each floor is lumped at the floor and roof diaphragm.

When assuming rigid diaphragms and inextensible columns, the multi story building is modeled as a shear building with lateral displacement of the lumped masses as the only degrees of freedom.

The dynamic response of the system is represented by the lateral displacement of the lumped masses with the number of degrees of dynamic freedom, or modes of vibration,  $n$ , being equal to the number of masses.

The resultant vibration of the system is given by the superposition of the vibrations of each lumped mass. Each individual mode of vibration has its own period and may be represented by a single degree of freedom system of the same period (See Fig. 3.5). Also, each mode shape remains of constant relative shape regardless of the amplitude of displacement.



**Fig. 3.5 Multiple Degrees of Freedom System**

Figure 3.5 shows the 4 modes of vibration of the 4 story shear building.

- The mode of vibration with the longest period (lowest frequency) is termed the first fundamental mode (Mode 1 has the longest period,  $T_1$ ).
- The other modes with shorter period of vibration (higher frequencies) are termed higher modes (Mode 2, 3, and 4 are higher frequency modes).

A Modal Analysis procedure is used to determine the dynamic response of a multiple degree of freedom structure. The maximum response for the separate modes is determined by modeling each mode as an individual single degree of freedom system. As the maximum values cannot all occur simultaneously, the maximum values are combined statistically in order to obtain the total response. The square root of the sum of squares is one method for combining the different modes maximums.

It is to be noted that the higher modes does not contribute significantly to the total response. An acceptable procedure is to utilize sufficient modes to ensure that 90% of the participating mass of the structure is included in the analysis. Typically, only few modes are sufficient to obtain the total dynamic response.

## RESPONSE SPECTRA

The response of a structure or a SDOF system (as shown on Fig. 3.1) to an earthquake ground motion depends on the natural period of vibration of the structure and its damping ratio.

- The maximum acceleration experienced by the structure due to a specific ground motion is known as **Spectral Acceleration,  $S_a$** .
- The maximum velocity experienced by the structure due to a specific ground motion is known as **Spectral Velocity,  $S_v$** .
- The maximum displacement experienced by the structure due to a specific ground motion is known as **Spectral Displacement,  $S_d$** .

The three spectral responses are interrelated using Newton's Second Law:

$$m S_a = k S_d$$

Since  $\omega^2 = k / m$

$$S_a = \omega^2 S_d$$

and

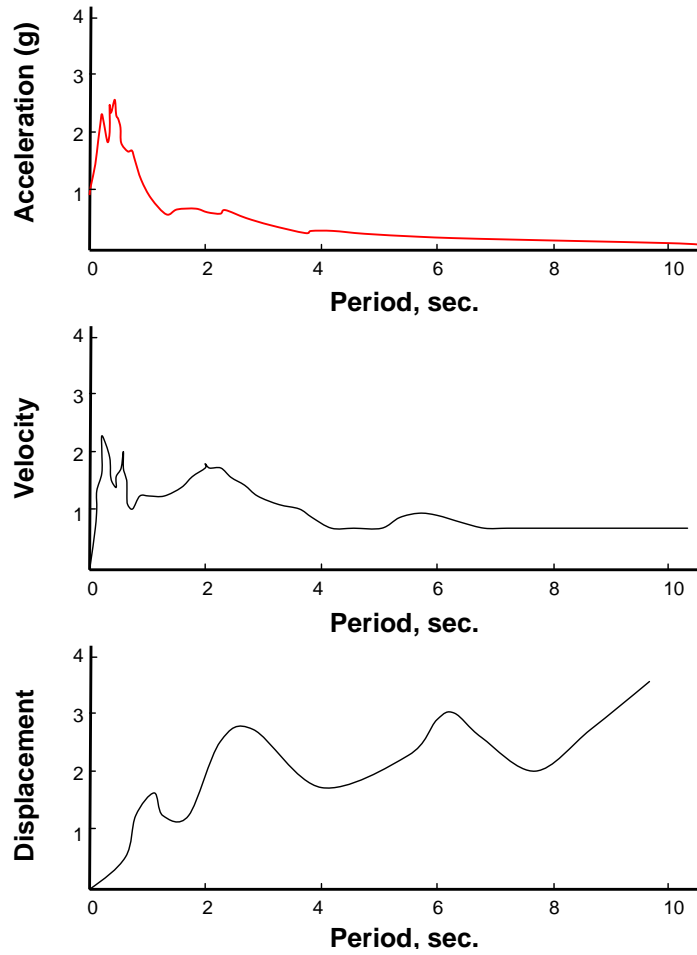
$$S_v = \omega S_d$$

|                                   |                   |
|-----------------------------------|-------------------|
| $S_a = \omega S_v = \omega^2 S_d$ | (undamped system) |
|-----------------------------------|-------------------|

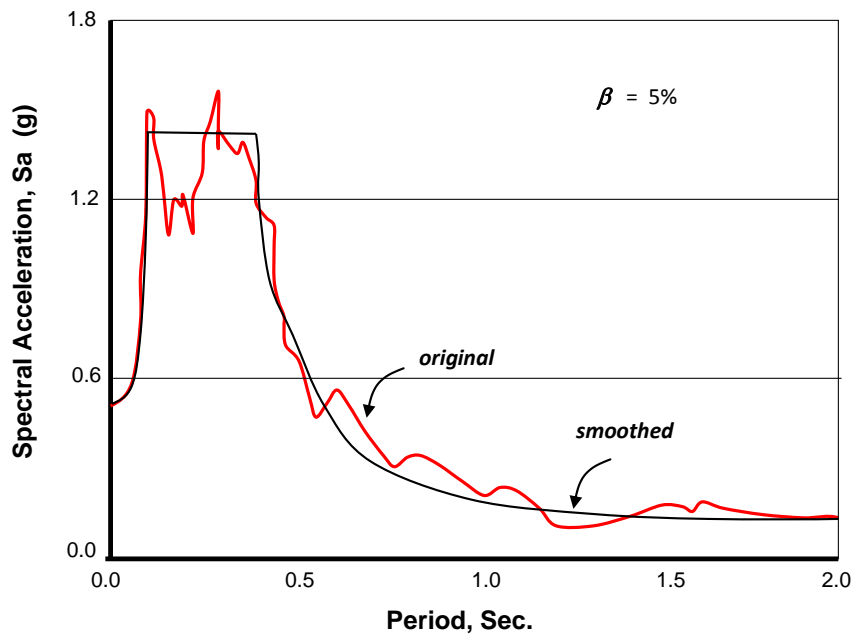
The three spectral values represent the maximum response for a SDOF with a certain mass, stiffness and hence natural period,  $T$ . By varying the stiffness or mass of the SDOF system and hence the natural period, a plot of any of the three spectral values against various natural periods can be obtained due to a specific ground motion and a certain damping ratio. The resulting plot summarizes the peak (spectral) response of all possible SDOF systems to a particular ground motion, and is called **response spectrum**. Figure 3.6 shows an example acceleration, velocity and displacement response spectrum for a structure with 5% damping ratio<sup>2</sup>.

- Response spectrum is a plot of all maximum responses (acceleration/ velocity/ displacement) of a structure with certain damping ratio to a particular ground motion.

Original response spectrum curves have random irregularities that could cause large variations in response for a slight change in period. For this reason, response spectrum are often smoothed (idealized) as shown on Fig. 3.7.

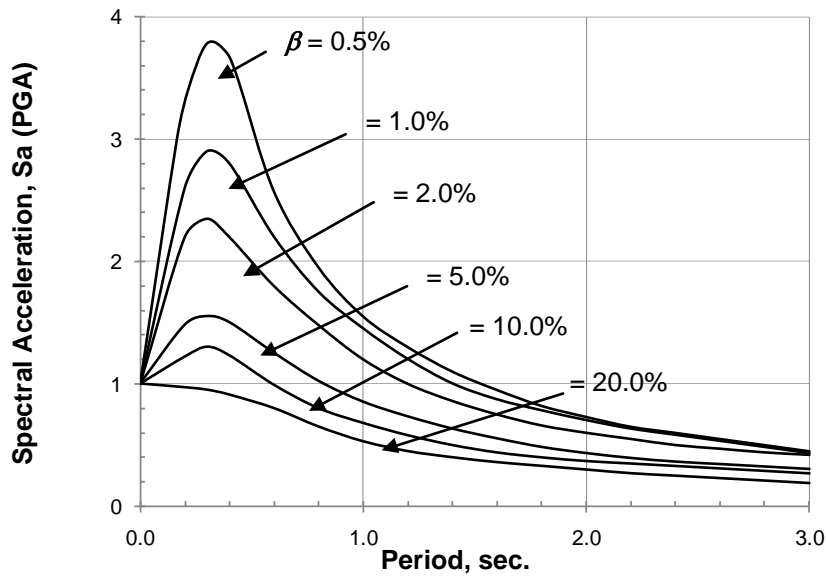


**Fig. 3.6** Acceleration, Velocity and Displacement Response Spectrum



**Fig. 3.7** Smoothed Acceleration Response Spectrum

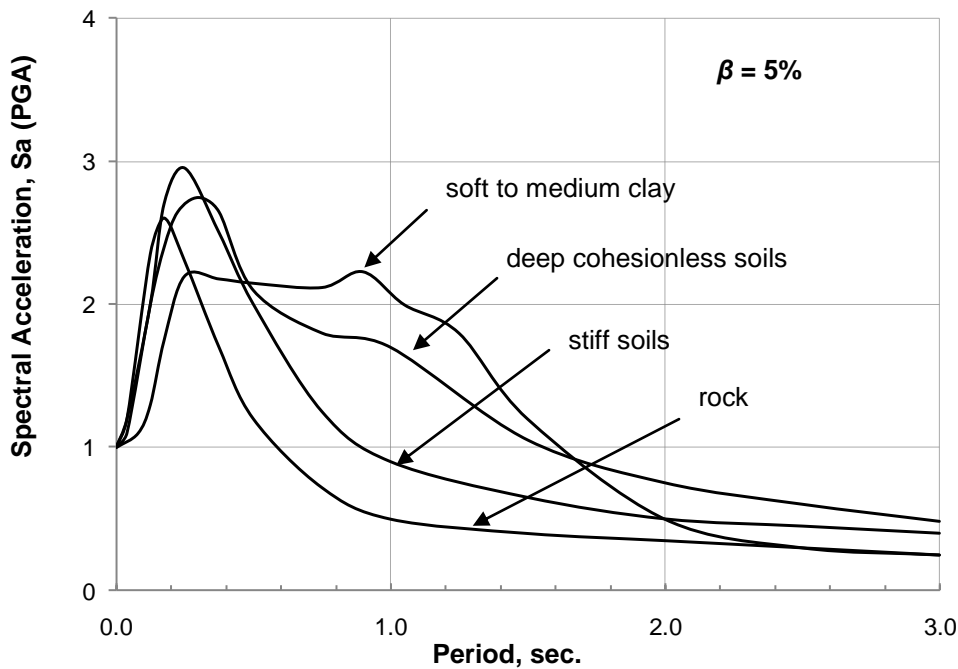
A *response spectra*, is a collection of *response spectrum* for variety of damping ratios and/or soil types. Figure 3.8 shows different acceleration response spectrum for variety of damping ratios. It is evident that by increasing the structural damping, the spectral values decrease<sup>4</sup>.



**Fig. 3.8 Acceleration Response Spectra for Variety of Damping ratios**

In addition to the damping ratio, soil conditions affect significantly the response of structures. Figure 3.9 shows acceleration response spectra for different soil conditions.

- The softer the soil, the longer the period of the structure that experiences the maximum acceleration response<sup>4</sup>.



**Fig. 3.9 Response Spectra for Different Soil Conditions**

Please note that Fig. 3.8 and 3.9 are normalized with respect to the effective peak ground acceleration.

### DESIGN RESPONSE SPECTRA

The response spectra shown previously were obtained for a ground motion specific to a particular accelerogram. For design purposes, the response curve must be representative of the characteristics of all seismic events that may occur at a particular site. Thus, several spectrum are developed for the site resulting from the different ground motions expected. The resulting spectra are then averaged and smoothed to eliminate any random irregularities (peaks and valleys)<sup>4</sup>.

Since soil conditions at a site substantially affect spectral shape, separate response curves are required for each representative soil type as shown on Fig. 3.10.

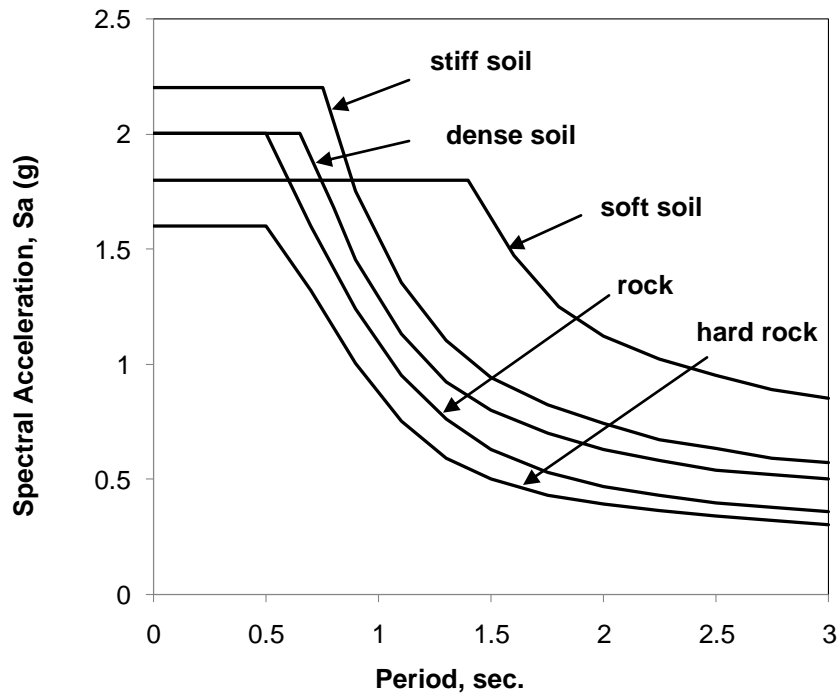


Fig. 3.10 Design Response Spectra for Different Soil Types

### BASE SHEAR

In summary, the maximum force imparted to the structure from an earthquake, *Base Shear*,  $V$ , is given by:

$$V = m S_a = W S_a / g$$

The base shear,  $V$ , can also be defined in the form of a coefficient/fraction,  $C_s$ , of the weight of the structure:

$$V = C_s W$$

It is to be noted that the above spectral relationship are approximate for damped system and for MDOF systems.

**Example 3.1:**

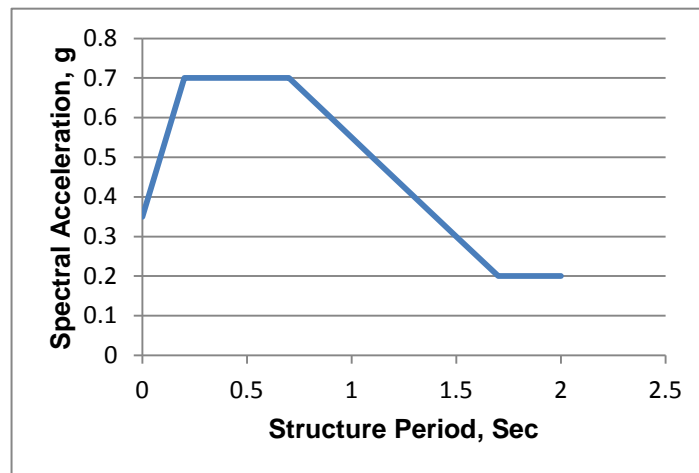
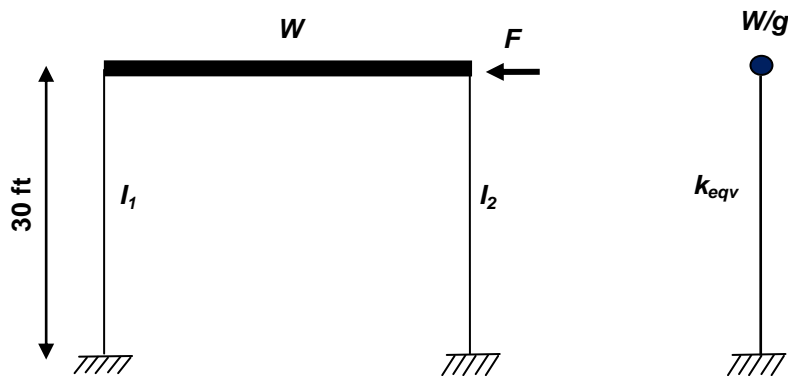
Figure shows a SDOF system, and the Design Response Spectrum where:

$W = 860$  kips

$E = 29,000$  ksi

$I_1 = I_2 = 22000$  in<sup>4</sup>

$S = 2800$  in<sup>3</sup>



Design Response Spectrum

**Determine Equivalent Stiffness of the SDOF system?**

Structure shown can be assumed as SDOF system with lumped mass,  $m = W/g$ , and equivalent stiffness  $k_{eqv}$ .

$$k_{eqv} = 12EI_1/H^3 + 12EI_2/H^3 = 12 \cdot 29000 \cdot 22000 / (30 \cdot 12)^3 + 12 \cdot 29000 \cdot 22000 / (30 \cdot 12)^3 = 328 \text{ k/in.}$$

**Determine natural period and natural frequency of the SDOF system?**

Natural period,  $T = 2\pi \sqrt{(W/k_{eqv}.g)} = 2*3.14*\sqrt{(860/328*386)} = 0.52 \text{ sec.}$

Frequency,  $f = 1/T = 1/0.52 = 1.93 \text{ Hz.}$

**Determine Base shear V of the SDOF system?**

From design response spectrum shown, for  $T=0.52 \text{ sec.}$ ,  $S_a = 0.7g$

Base Shear,  $V = m*S_a = W*S_a/g = 860*0.7g / g = 602 \text{ kips}$

**Determine lateral deflection Δ of the SDOF system?**

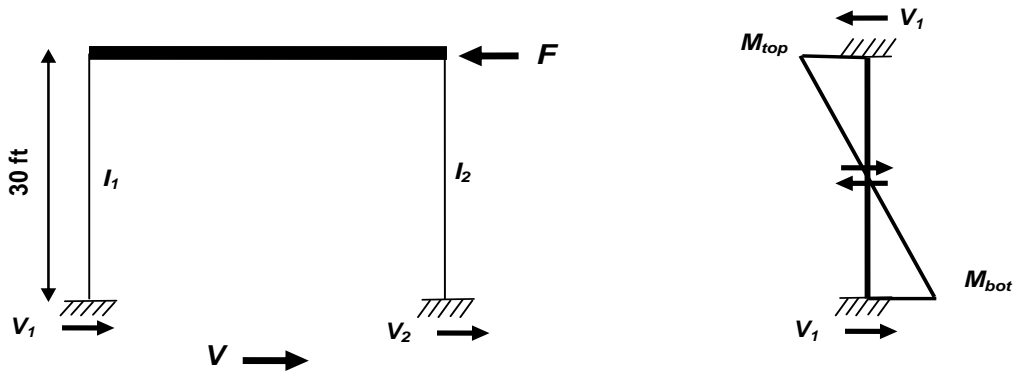
The total base shear V determined above (602 k) equals the lateral force,  $F$ , applied at the lumped mass.

Deflection,  $\Delta = F/k_{eqv} = 602/328 = 1.83 \text{ in.}$

**Determine shear force in Column 1?**

Base shear will be divided between columns based on their relative stiffness. Since  $I_1 = I_2$ . Thus  $V_1 = V_2 = 602/2 = 301 \text{ kips}$ .

**Determine maximum bending moment in Column 1?**



$$M_{top} = M_{bot} = V_1 * H/2 = 301*30/2 = 4515 \text{ k.ft}$$

**Determine maximum bending stress in Column 1?**

Bending Stress at bottom (or top),  $\sigma = M_{bot}/S = 4515/2800 = 19.4 \text{ ksi}$

**Example 3.2:**

Figure shows two SDOF systems, and their Design Response Spectrum. Assume Fixed-Pinned conditions.

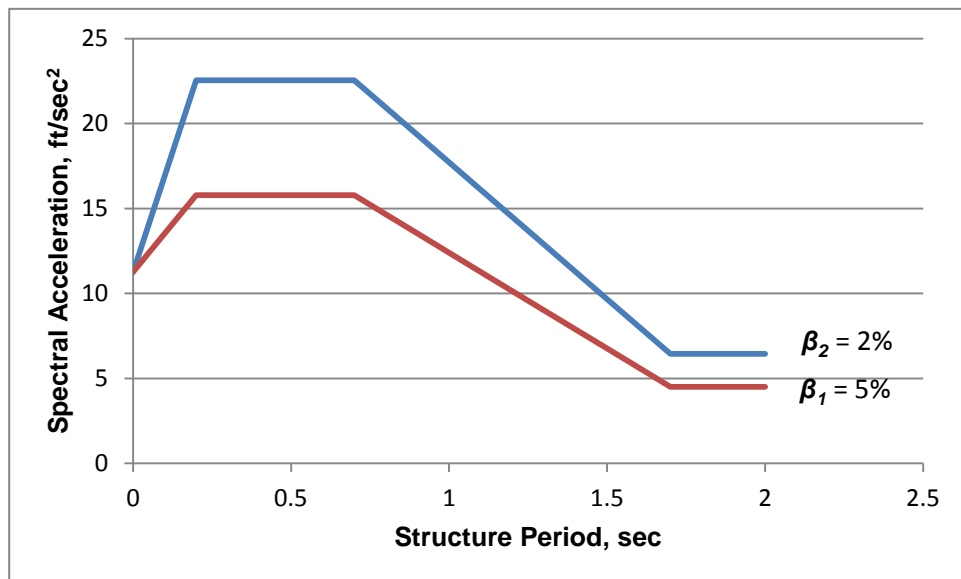
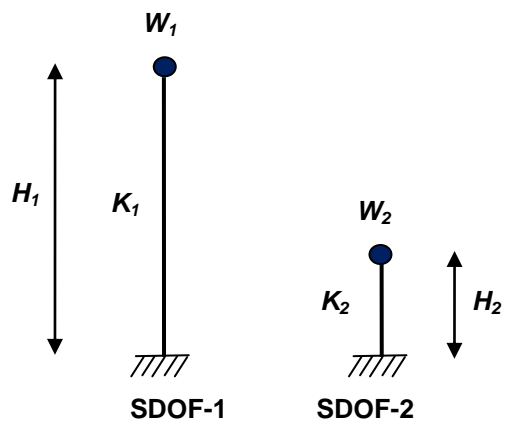
$$W_1 = 2 W_2$$

$$E_1 = E_2 = E$$

$$I_1 = 4 I_2$$

$$H_1 = 3 H_2$$

$$\beta_1 = 5\% \quad \beta_2 = 2\%$$



Design Response Spectrum

**Determine the stiffness ratio of SDOF-2 to SDOF-1 ?**

$$K_2 = 3 * E * I_2 / (H_2)^3$$

$$K_1 = 3 * E * I_1 / (H_1)^3 = 3 * E * 4 * I_2 / (3 * H_2)^3 = 3 * 4 * E * I_2 / (27 * H_2^3) = (4/27) K_2$$

$$= 0.148 K_2$$

$$K_2 = (27/4) K_1 = 6.75 K_1$$

*(The shorter the structure, the stiffer it gets)*

**Determine the natural period ratio of SDOF-1 to SDOF-2 ?**

$$K_2 = (27/4) K_1 = 6.75 K_1 \quad W_1 = 2 W_2$$

$$T_1 = 2\pi \sqrt{\frac{W_1}{k_1 * g}} \quad T_2 = 2\pi \sqrt{\frac{W_2}{k_2 * g}}$$

$$\frac{T_1}{T_2} = \frac{\sqrt{W_1} \sqrt{k_2}}{\sqrt{W_2} \sqrt{k_1}}$$

$$\frac{T_1}{T_2} = \frac{\sqrt{2 W_2} \sqrt{6.75 k_1}}{\sqrt{W_2} \sqrt{k_1}} = \sqrt{2} \sqrt{6.75} = 3.67$$

$$T_1 = 3.67 T_2 \text{ and } T_2 = 0.27 T_1$$

*(the taller the structure, the longer the natural period)*

**Determine the base shear ratio of SDOF-2 to SDOF-1 ? Assume  $T_1 = 1.0$  sec**

From Design Response Spectrum

$$\text{SDOF-1, } \beta_1 = 5\% \quad T_1 = 1.0 \text{ sec, } S_{a1} = 12.5 \text{ ft/sec}^2$$

$$\text{SDOF-2, } \beta_2 = 2\% \quad T_2 = 0.27 \text{ sec, } S_{a2} = 22.5 \text{ ft/sec}^2$$

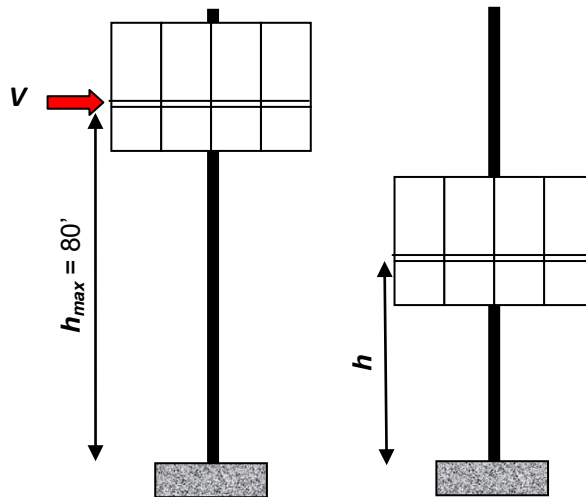
$$V_1 = W_1 * S_{a1} / g = W_1 * 12.5 / 32.2 = 0.39 * W_1 = 0.39 * 2 * W_2 = 0.78 * W_2$$

$$V_2 = W_2 * S_{a2} / g = W_2 * 22.5 / 32.2 = 0.70 * W_2$$

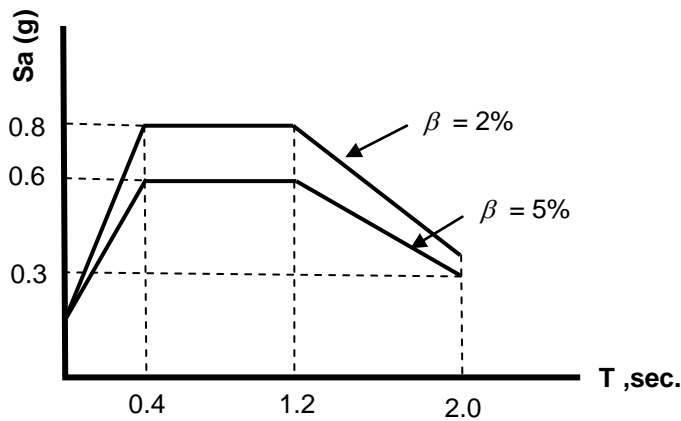
$$V_2 / V_1 = 0.70 / 0.78 = 0.90 \quad V_2 = 0.90 V_1$$

**Example 3.3:**

The amusement lift shown in Figure moves contenders up and down for different heights at different speed. The lift can be approximated as a single degree of freedom system with 5% critical damping. Design Response spectrum is shown. Table illustrates the lift system's natural period of vibration corresponding with the maximum height of the desired lift. What is the maximum overturning moment at the base of the lift? Assume weight of the lift and contenders equals 15 kips



| Height, $h$<br>ft | Time, $T$<br>Sec. |
|-------------------|-------------------|
| 80                | 2.0               |
| 60                | 1.6               |
| 40                | 1.2               |
| 20                | 0.8               |



**Determine the maximum overturning moment at the base of the lift:**

Overtuning Moment at the Base:  $OTM = V * height$

$$\begin{aligned} \text{Base shear: } V &= m S_a = W S_a / g \\ &= 15 \text{ kips/g} * S_a(g) = 15 \text{ kips} * S_a \end{aligned}$$

Since the lift oscillates up to different heights at different speed. Reading the spectral acceleration,  $S_a$ , from the design response spectrum given for 5% critical damping for each lift height,  $h$ , according to its period of vibration (oscillation),  $T$ .

| Height, $h$<br>ft | Time, $T$<br>Sec. | $S_a$<br>(g) | $W * S_a * h$<br>kips.ft                 |
|-------------------|-------------------|--------------|--|
| 80                | 2.0               | 0.3          | $15 * 0.3 * 80 = 360$                    |
| 60                | 1.6               | 0.45         | <b><math>15 * 0.45 * 60 = 405</math></b> |
| 40                | 1.2               | 0.6          | $15 * 0.6 * 40 = 360$                    |
| 20                | 0.8               | 0.6          | $15 * 0.6 * 20 = 180$                    |

Maximum Overtuning Moment at the Base = 405 kips.ft

## MULTIPLE CHOICE QUESTIONS

**3.1 Member's rigidity can be best described as?**

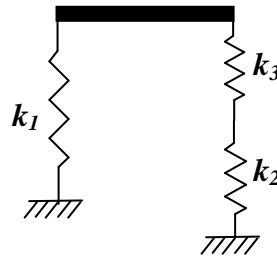
- A the inverse of member's deflection
- B the inverse of member's stiffness
- C the inverse of member's ductility
- D the inverse of member's damping

**3.2 For a SDOF system, when the mass,  $m$ , increases, what is the effect on natural period,  $T$ , and base shear,  $V$  ?**

- A  $T$  decreases and  $V$  increases
- B  $T$  decreases and  $V$  decreases
- C  $T$  increases and  $V$  increases
- D  $T$  increases and  $V$  decreases

**3.3 Determine the equivalent stiffness of the SDOF system shown in the figure? Where  $k_1 = 80$  kips/in,  $k_2 = 60$  kips/in, and  $k_3 = 40$  kips/in.**

- A 27 kips/in
- B 34 kips/in
- C 104 kips/in
- D 180 kips/in



**3.4 What is the spectral velocity of a single degree of freedom system?**

- A the minimum velocity experienced by the structure due to a specific ground motion
- B the average velocity experienced by the structure due to a specific ground motion
- C the maximum acceleration experienced by the structure due to a specific ground motion multiplied times angular frequency
- D the maximum displacement experienced by the structure due to a specific ground motion multiplied times angular frequency

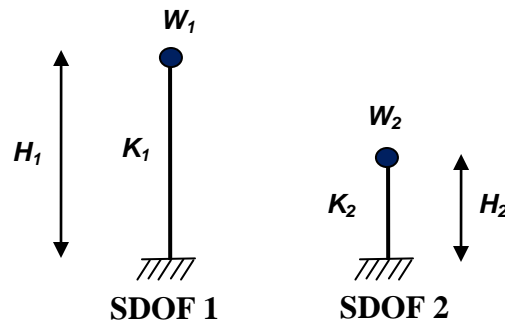
**3.5 Maximum acceleration experienced by the building from a specific ground motion is defined as?**

- A peak ground acceleration
- B dynamic acceleration
- C spectral acceleration
- D maximum building velocity relative to ground acceleration

**3.6 Determine the natural period of SDOF 2? Assume Fixed-Fixed condition.**

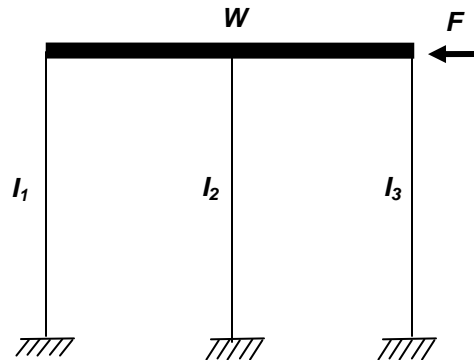
$$W_1 = W_2 = W; E_1 = E_2 = E; I_1 = 2I_2; H_1 = 2H_2$$

- A  $0.50 T_1$
- B  $0.85 T_1$
- C  $2.50 T_1$
- D  $3.50 T_1$



**3.7 For the structure shown subjected to lateral force,  $F$ , determine the ratio of shear forces distributed to the three columns, respectively? The three columns have the same height, modulus of elasticity and boundary conditions. Assume column's second moment of inertia are distributed as  $4I_1 = 2I_2 = I_3$ .**

- A  $1/7 F, 2/7 F, 4/7 F$
- B  $4/7 F, 2/7 F, 1/7 F$
- C  $1/3 F, 1/3 F, 1/3 F$
- D  $-4/6 F, -2/6 F, 1.0 F$



**3.8 Natural period,  $T$ , of a SDOF system is determined from:**

- A the structure response spectrum
- B equivalency to the system's linear frequency
- C the inverse of the system's angular frequency
- D system's mass and stiffness

- 3.9 What is the damping of an oscillating SDOF system?**
- A shortest time between successive cycles of vibration
  - B energy modification response factor
  - C rate of change of displacement amplitude
  - D decay of vibration amplitude with time
- 3.10 For a Multiple Degrees Of Freedom system, MDOF, the term “higher modes” refers to?**
- A modes of vibration with the longest periods
  - B modes of vibration with the shortest periods
  - C modes of vibration with the shortest frequencies
  - D modes of vibration with the highest participation factor
- 3.11 Critical damping of an oscillating harmonic system can be best described as the .....**
- A damping to bring the harmonic system to static position in the shortest possible time
  - B ratio of actual damping to critical mass of the system
  - C decay of vibration amplitude with time
  - D factor of vibration underdamped amplitude to overdamped amplitude
- 3.12 Determine the spectral displacement,  $S_d$ , of a SDOF system that has natural period of 0.5 sec, and a spectral acceleration,  $S_a$ , of 0.75g?**
- A 1.54 in.
  - B 1.84 in.
  - C 2.94 in.
  - D 4.75 in.
- 3.13 Natural period of a SDOF system can be best described as?**
- A the fundamental period of vibration
  - B the inverse of natural frequency
  - C the time between two successive peaks or valleys
  - D all of the above descriptions

**3.14 The response spectrum is best described as .....**

- A a graph of the maximum responses of SDOF systems to a specified excitation
- B the maximum response by a SDOF system to a specified excitation
- C a collection of several response spectra
- D the maximum ground response of SDOF system to a specified excitation

**3.15 The dynamic response of a MDOF system can be best represented by .....**

- A the fundamental mode of vibration
- B sufficient modes of vibration that contribute more than 90% of the participating mass
- C the square root of sum of squares of the first three modes of vibration
- D all of the above descriptions

**Refer to the building shown in the Figure for MCQs 3.16 to 3.20**

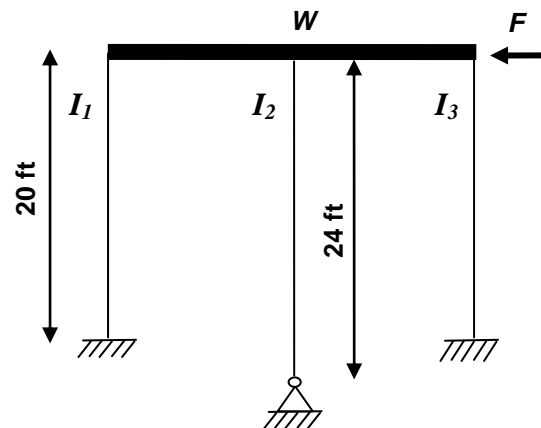
$W = 1000$  kips

$E = 29000$  ksi

$I_1 = I_3 = 12000$  in<sup>4</sup>

$I_2 = 15000$  in<sup>4</sup>

$S_a = 0.75g$



**3.16 Determine the equivalent stiffness of the SDOF system?**

- A 360 k/in
- B 650 k/in
- C 660 k/in
- D 825 k/in

**3.17 Determine the natural period of the SDOF system?**

- A 0.35 sec
- B 0.39 sec
- C 0.40 sec
- D 0.54 sec

**3.18 Determine the lateral deflection of the SDOF system ?**

- A 0.91 in.
- B 1.14 in.
- C 1.16 in.
- D 2.10 in.

**3.19 Determine the shear in Column 2 of the SDOF system ?**

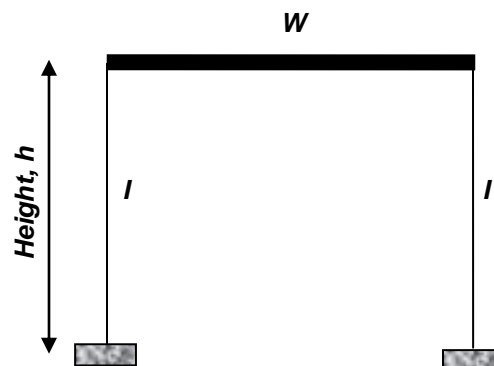
- A 62 kips
- B 249 kips
- C 344 kips
- D 688 kips

**3.20 Determine the flexural stress in Column 2 of the SDOF system, given sectional modulus,  $S = 1000 \text{ in}^3$  ?**

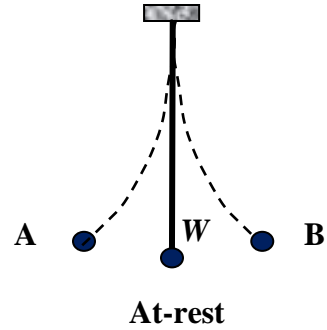
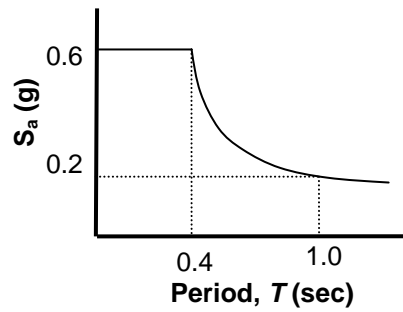
- A 12 ksi
- B 16 ksi
- C 18 ksi
- D 36 ksi

**3.21 The SDOF structure shown has a period of vibration equal 0.8 sec. Which of the following best describes the change in period of vibration if the height of the structure,  $h$ , increases by 60%?**

- A period will increase by a factor of 2
- B period will decrease by a factor of 1.6
- C period will increase by a factor of 1.6
- D period will increase by a factor of 1.3

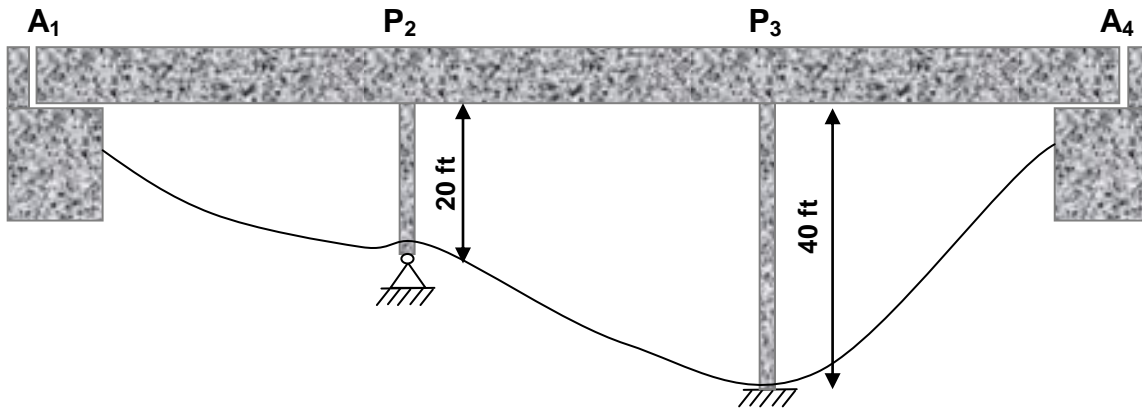


**A massless beam with a fixed support is hanging vertically with a weight,  $W = 5$  kips, attached to its free end. The beam has a height of 10 ft and takes 0.2 seconds to oscillate from at rest position to either positions A or B. Answer MCQs 3.22 to 3.25.**



- 3.22 Determine the natural period of vibration of the hanging beam?**
- A 0.2 sec
  - B 0.4 sec
  - C 0.8 sec
  - D 1.6 sec
- 3.23 If the period of vibration is 1.0 sec., which of the following represents the moment at the fixed base?**
- A 0.31 kips.ft
  - B 1.0 kips.ft
  - C 5.0 kips.ft
  - D 10 kips.ft
- 3.24 If the height of the beam is reduced to 6 ft, and the period of vibration is reduced to 0.4 sec., which of the following represents the moment at the fixed base?**
- A 0.56 kips.ft
  - B 1.0 kips.ft
  - C 18.0 kips.ft
  - D 30.0 kips.ft
- 3.25 Which of the following statements best represents the maximum moment at the fixed base of the hanging beam?**
- A function of highest acceleration and longest beam height
  - B function of the highest acceleration and shortest beam height
  - C function of the smallest acceleration and longest beam height
  - D none of the above

A concrete bridge overcrossing a valley with variable height piers is shown in the Figure. The seismic force is determined to be 1800 kips resisted only by the two piers,  $P_2$  and  $P_3$ . Abutment 1 and 4 do not resist the seismic loads. Weight of the bridge is 8000 kips.  $E = 4000$  ksi. Inertia of  $P_2 = P_3 = 15.4 \text{ ft}^4$ . Answer MCQs 3.26 to 3.28.



**3.26 Determine the seismic force resisted by Pier  $P_2$ ?**

- A 600 kips
- B 800 kips
- C 1200 kips
- D 1500 kips

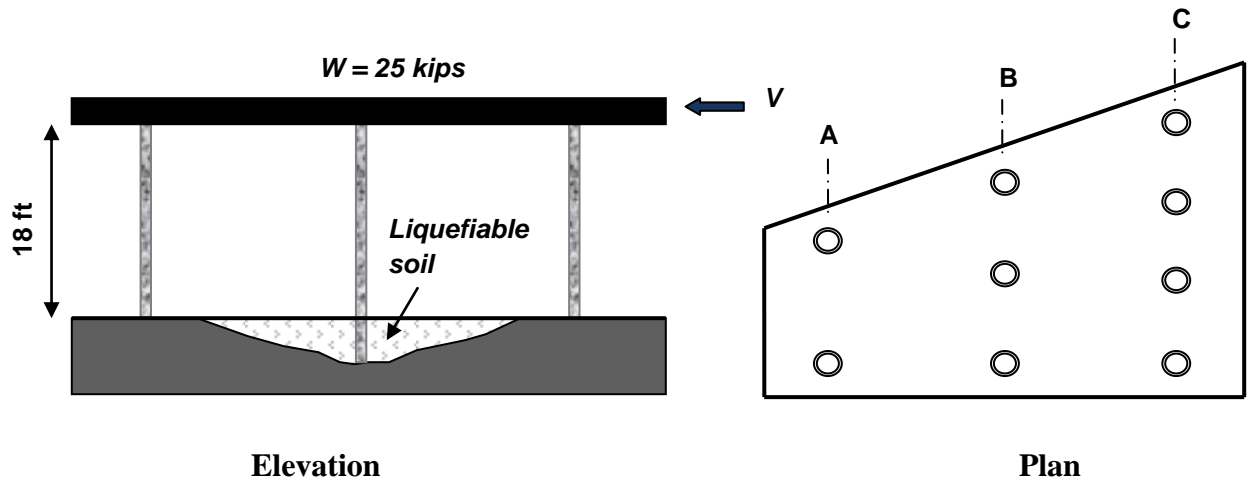
**3.27 Determine the fundamental period of vibration for the bridge structure?**

- A 0.6 sec
- B 0.8 sec
- C 1.0 sec
- D 1.4 sec

**3.28 Determine the deflection at top of Pier  $P_3$ ?**

- A 1.20 in.
- B 3.67 in.
- C 4.33 in.
- D 6.49 in.

A one story factory building shown in the Figure can be approximated as a Single Degree of Freedom System. The building has 9 columns all made of hollow precast concrete ( $E = 3800$  ksi,  $I = 1816$  in<sup>4</sup>). All columns are assumed pinned at the top and fixed at the bottom (top of soil profile). Columns along line B is embedded in liquefiable soil and the fixity can be assumed 6 ft below the top of soil. Ignore any torsional effect. Answer MCQ's 3.29 to 3.34.



**3.29 Determine the total equivalent stiffness of the building?**

- A 0.86 k/in
- B 2.05 k/in
- C 14.93 k/in
- D 18.48 k/in

**3.30 Which of the following represents the fundamental period of vibration of the building?**

- A 0.25 sec
- B 0.32 sec
- C 0.41 sec
- D 0.66 sec

**3.31 Determine the seismic base shear,  $V$ , given a spectral acceleration,  $S_a = 0.6g$  ?**

- A 12 kips
- B 15 kips
- C 20 kips
- D 22 kips

**3.32** Given a base shear,  $V$ , of 20 kips. Determine the deflection,  $\Delta$ , at the roof level?

- A 0.60 in.
- B 0.87 in.
- C 1.02 in.
- D 1.34 in.

**3.33** Determine the shear force resisted by the middle column along line B, given a base shear,  $V$ , of 30 kips?

- A 1.74 kips
- B 3.22 kips
- C 5.23 kips
- D 10.0 kips

**3.34** Determine the percentage of the base shear,  $V$ , that would be resisted by the four columns along line C, given a base shear,  $V$ , of 40 kips?

- A 14%
- B 27%
- C 55%
- D 67%

**3.35** Two separate SDOF buildings, building A and building B. The weight of building A is twice the weight of building B. The period of vibration of building A is twice the period of vibration of building B. Which of the following statement is correct regarding the stiffness of the two buildings,  $k_A$  and  $k_B$ ?

- A  $k_A = 2 * k_B$
- B  $k_B = 2 * k_A$
- C  $k_A = 4 * k_B$
- D  $k_B = \sqrt{2} * k_A$